

PROJECT SCOPE

1. STRUCTURAL SCOPE IS LIMITED TO ANALYSIS OF EXISTING BUILDING FOR LOADS IMPARTED BY NEW MECHANICAL AND ELECTRICAL EQUIPMENT, NEW MECHANICAL DUCTWORK AND DESIGN OF REQUIRED STRENGTHENING DETAILS.

GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL, PREPARED AND SUBMITTED BY THE CONTRACTOR.
3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS, WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION, FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.
8. CONTRACTOR SHALL VERIFY THE STRUCTURALLY SUPPORTED MECHANICAL AND ELECTRICAL EQUIPMENT WEIGHTS, OPENING SIZES AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
9. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL AND ELECTRICAL EQUIPMENT IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.
10. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
11. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
12. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
13. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
14. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
15. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.
16. SUBMITTALS
- 16.1 SUBMITTALS BY THE CONTRACTOR ARE NOT A PART OF THE CONTRACT DOCUMENTS. PRIOR TO THE INITIAL SUBMITTAL, CONTRACTOR SHALL SUBMIT TO THE DESIGN PROFESSIONAL A SCHEDULE OF SUBMITTED INFORMATION.
- 16.2 SUBMITTALS SHALL BE ACCOMPANIED BY A TRANSMITTAL LETTER WITH THE FOLLOWING INFORMATION:
- PROJECT NAME
 - CONTRACTOR'S NAME
 - DATE SUBMITTED
 - DESCRIPTION OF ITEMS SUBMITTED, IDENTIFY WORK AND PRODUCT BY SPECIFICATION SECTION
 - NUMBER OF DRAWINGS AND OTHER PERTINENT DATA.
- 16.3 CONTRACTOR SHALL DIRECT SPECIFIC ATTENTION ON THE SUBMITTAL TO ANY DEVIATION FROM THE CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP AND SIGN EACH SHEET OF SHOP DRAWINGS AND PRODUCT DATA, AND SIGN OR INITIAL EACH SAMPLE TO CERTIFY COMPLIANCE WITH REQUIREMENTS OF CONTRACT DOCUMENTS. SUBMITTALS RECEIVED WITHOUT THE CONTRACTOR'S STAMP OF REVIEW WILL BE RETURNED TO THE CONTRACTOR FOR REVIEW AND RESUBMITTAL.
- 16.4 WORK REQUIRING SHOP DRAWINGS, WHETHER CALLED FOR BY THE CONTRACT DOCUMENTS OR REQUESTED BY THE CONTRACTOR, SHALL NOT COMMENCE UNTIL THE SUBMISSION HAS BEEN REVIEWED BY THE DESIGN PROFESSIONAL. WORK MAY COMMENCE IF THE CONTRACTOR VERIFIES THE ACCURACY OF THE DESIGN PROFESSIONAL'S CORRECTIONS AND NOTATIONS AND COMPLES WITH THEM WITHOUT EXCEPTION AND WITHOUT REQUESTING CHANGE IN CONTRACT SUM OR CONTRACT TIME AT COPY OF THE MARKED STRUCTURAL SHOP DRAWINGS WITH THE DESIGN PROFESSIONAL'S REVIEW STAMP IS TO BE MAINTAINED AT THE JOB SITE.

CODE/DESIGN CRITERIA

1. STRUCTURAL COMPONENTS ARE DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
- INTERNATIONAL BUILDING CODE, 2018 EDITION WITH GEORGIA AMENDMENTS.
2. GRAVITY LOADS
- 2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
- GENERAL OFFICE AREAS 50 PSF
 - LOBBIES AND FIRST FLOOR CORRIDORS 100 PSF
 - CORRIDORS ABOVE FIRST FLOOR 80 PSF
 - ELECTRICAL AREAS 125 PSF
 - MECHANICAL AREAS 100 PSF
- 2.2 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):
- FLOOR:
- MISCELLANEOUS 5 PSF
 - CEILING/MEP 10 PSF
3. EARTHQUAKE LOADS:
- RISK CATEGORY: II
 - SEISMIC IMPORTANCE FACTOR: I = 1.0
 - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, $S_s = 0.24$
 - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, $S_1 = 0.093$
 - SITE CLASS D (ASSUMED)
 - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, $S_{DS} = 0.21$
 - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, $S_{D1} = 0.13$
 - SEISMIC DESIGN CATEGORY: B
4. THE EXISTING STRUCTURE, WITH PROPOSED MODIFICATIONS, HAS BEEN ANALYZED FOR GRAVITY AND LATERAL LOADS AND FOUND TO BE IN COMPLIANCE WITH IBC 2012 SECTION 3403 AND 3404 FOR ADDITIONS AND ALTERATIONS TO AN EXISTING STRUCTURE.

5. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:
- FLOOR MEMBERS:
- DEAD LOAD L_{D60}
- LIVE LOAD L_{L60}
- DEAD + LIVE LOAD $L_{D60} + L_{L60}$
- WHERE L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS, (FOR CANTILEVERS L IS TWICE THE LENGTH OF THE CANTILEVER) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFLECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.
 - THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED $L/600$ FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.
6. SPECIAL INSPECTIONS:
- 6.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 014525, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE STEEL CONSTRUCTION. SEE SPECIFICATION SECTIONS 014525 FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.
- 6.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO STRUCTURAL STEEL FRAMING.
- 6.3 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
- 6.4 THE PROJECT OWNER WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
- 6.5 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE DESIGN PROFESSIONAL WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.
- 6.6 SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.
7. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
- STRUCTURAL STEEL HSS SHAPES SHALL CONFORM TO ASTM A500, GRADE C.
 - MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
2. BOLTS AND ANCHORS:
- 2.1 BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER F3125 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2014 (SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS).
- 2.2 EXPANSION ANCHORS SHALL HAVE BEEN EVALUATED BY THE ICC EVALUATION SERVICES, INC (ICC-ES) WITH A PUBLISHED EVALUATION REPORT. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 193 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. ALL ANCHORS SHALL BE APPROVED FOR RESISTING WIND AND SEISMIC LOADS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
- 2.3 ADHESIVE ANCHORS SHALL CONSIST OF AN ALL-THREAD STEEL ANCHOR WITH ADHESIVE CONFORMING TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERRECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION OF 10 KIPS UNLESS SHOWN OTHERWISE ON THE STRUCTURAL DRAWINGS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. DESIGN PROFESSIONAL SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR. STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS FOR SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPICTING ARRANGEMENT CONCEPT OF THE CONNECTION WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.
5. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY D1.1/D1.1M-2015. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
6. STEEL BAR GRATING SHALL CONFORM TO THE FOLLOWING, UNLESS NOTED OTHERWISE:
- 6.1 STEEL BAR GRATING SHALL BE RECTANGULAR TYPE WITH WELDED CROSS BARS.
- 6.2 BEARING BARS SHALL BE 1-1/2" DEEP X 3/16" THICK SPACED AT 1-3/16" C/C WITH CROSS BARS SPACED AT 4" C/C. STEEL BAR MATERIAL SHALL CONFORM TO ASTM A569.
- 6.3 GRATING FINISH SHALL BE PLAIN.
- 6.4 GRATING FINISH TO BE STANDARD SHOP COAT PAINT.
- 6.5 FASTEN GRATING TO STEEL SUPPORTS WITH SADDLE CLIP AND SELF-DRILLING FASTENER AT EVERY SIXTH BEARING BAR ALONG SUPPORT (MIN. OF 2 CLIPS PER PANEL).

ABBREVIATIONS

ABT	ABOUT	(E)	EXISTING	IF	INSIDE FACE
ACI	AMERICAN CONCRETE INSTITUTE	EA	EACH	IBC	INTERNATIONAL BUILDING CODE
ADDL	ADDITIONAL	EF	EACH FACE	ICC	INTERNATIONAL CODE COUNCIL
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	ELEV	ELEVATION	ID	INSIDE DIAMETER
ALT	ALTERNATE	ELEC	ELECTRICAL	IE	INVERT ELEVATION
ENGR	ENGINEER	IN	INCH	INT	INTERIOR
APPROX	APPROXIMATE	EOD	EDGE OF DECK	JT	JOINT
ARCH	ARCHITECTURAL/ARCHITECT	EOR	ENGINEER OF RECORD		
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	EOS	EDGE OF SLAB		
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	EQU	EQUAL	K	KIPS(S)
AWS	AMERICAN WELDING SOCIETY	EQUIP	EQUIPMENT	KSF	KIPS PER SQUARE FOOT
		EW	EACH WAY	KSI	KIPS PER SQUARE INCH
B	BOTTOM OF	EXP	EXPANSION	EXT	EXTERIOR
BLDG	BUILDING	FD	FLOOR DRAIN	(LLH)	LONG LEG HORIZONTAL (ANGLE)
BM	BEAM	(LDV)	LONG LEG VERTICAL (ANGLE)	(LSH)	LONG SIDE HORIZONTAL (HSS)
BOTT	BOTTOM	(LSV)	LONG SIDE VERTICAL (HSS)	LB	POUND
BRG	BEARING	FF	FINISHED FLOOR	LF	LINEAR FEET
BTWN	BETWEEN	FG	FINISHED GRADE	LL	LIVE LOAD
		FLG	FLANGE	LOC	LOCATION
C/C	CENTER TO CENTER	FRG	FRAMING	LONG	LONGITUDINAL
CALC	CALCULATIONS	FRP	FIBER REINFORCED PLASTIC	LP	LOW POINT
CHKD	CHECKED	FS	FAR SIDE	LSH	LONG SLOTTED HOLE
CIP	CAST-IN-PLACE CONCRETE	FT	FOOT	LWC	LIGHT WEIGHT CONCRETE
CJ	CONSTRUCTION CONTROL JOINT	FTG	FOOTING	M	MOMENT
CJP	COMPLETE JOINT PENETRATION	FV	FIELD VERIFY	MAX	MAXIMUM
CL	CENTERLINE	GA	GAGE, GAUGE	MC	MOMENT CONNECTION
CLR	CLEAR, CLEARANCE	GALV	GALVANIZED (HOT DIP)	MECH	MECHANICAL
COL	COLUMN	GRG	GRATING	MFG	MANUFACTURED
CONC	CONCRETE	(H)	HORIZONTAL BEAM ORIENTATION	MFR	MANUFACTURER
CONC	CONCRETE	HCA	HEADED CONCRETE ANCHOR	MIN	MINIMUM
COORD	COORDINATE	HDR	HEADER	MISC	MISCELLANEOUS
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	HOR	HANGER	MTD	MOUNTED
CTRD	CENTERED	HORIZ	HORIZONTAL	MTL	METAL
		HP	HIGH POINT	N&F	NEAR AND FAR
DBA	DEFORMED BAR ANCHOR	HR	HANDRAIL	N/A	NOT APPLICABLE
DBL	DOUBLE	HSB	HIGH STRENGTH BOLT	NIC	NOT IN CONTRACT
DEG	DEGREES			NO. NO.	NUMBER
DETAL	DETAIL			NOM	NOMINAL
DIA	DIAMETER			NS	NEAR SIDE
DIAG	DIAGONAL			NTS	NOT TO SCALE
DIR	DIRECTION			NWC	NORMAL WEIGHT CONCRETE
DL	DEAD LOAD				
DWG	DRAWING				

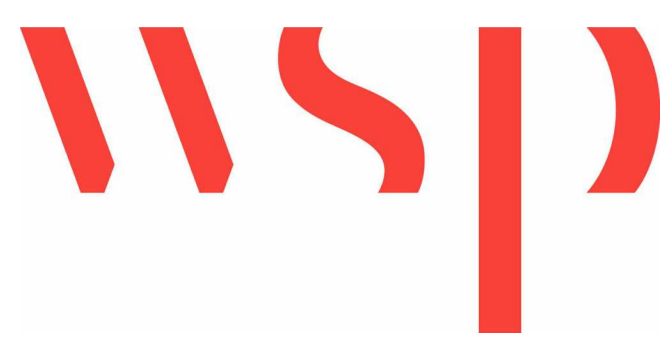
CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:
- 2.1 NORMAL WEIGHT STRUCTURAL CONCRETE:
- SLABS 4000 PSI
3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
4. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
5. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.
6. CONCRETE MIX DESIGN FOR 4000 PSI CONCRETE SHALL BE BASED ON A MAXIMUM AGGREGATE SIZE OF 1 IN. MAXIMUM WATER/CEMENT RATIO OF: 50 FOR NON-AIR-ENTRAINED CONCRETE AND 45 FOR AIR-ENTRAINED CONCRETE AND A MAXIMUM SLUMP OF 9 IN. AIR ENTRAINED CONCRETE SHALL BE USED FOR EXTERIOR EXPOSED CONCRETE WITH AN AIR CONTENT BETWEEN 3 AND 6 PERCENT, UNO.
7. CONCRETE SLABS ON GRADE SHALL NOT BE LOADED UNTIL A MINIMUM CONCRETE STRENGTH OF 1800 PSI HAS BEEN ATTAINED AND THE CONCRETE IS AT LEAST THREE DAYS OLD. ALL OTHER CONCRETE MEMBERS SHALL NOT BE LOADED UNTIL THE SPECIFIED CONCRETE DESIGN STRENGTH HAS BEEN ATTAINED.
8. CONCRETE SHALL BE TESTED IN ACCORDANCE WITH ACI 301 AND THE SPECIFICATIONS FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. AT A MINIMUM CONCRETE SPECIMENS SHALL BE TAKEN FOR EVERY 100 YARDS OR PORTION THEREOF FOR EACH MIX DESIGN PLACED IN A DAY. CONCRETE TEST REPORTS SHALL BE AVAILABLE ON SITE FOR INSPECTION.
9. UNLESS NOTED OTHERWISE, ALL REINFORCING SHALL BE CONTACT LAP SPICED WITH A CLASS B SPLICE IN ACCORDANCE WITH ACI 318-14. SPLICE LENGTHS SHALL BE INCREASED BY A FACTOR OF 1.3 FOR TOP REINFORCEMENT. LAP WELDED WIRE FABRIC (WWF) ONE SPACE PLUS 2 IN. ON ALL SIDES AT SPLICES. ALL BARS SHALL BE SUPPORTED BY BAR SUPPORTS CONFORMING TO CRSI SPECIFICATIONS. SUPPORT SPACING OF BARS SHALL NOT EXCEED 4 FEET. SUPPORT SPACING OF WWF SHALL NOT EXCEED 2 FEET.
10. ALL EXPOSED CORNERS OF CONCRETE SHALL HAVE A CHAMFER OR RADIUS OF 3/4", UNLESS NOTED OTHERWISE.
11. CONCRETE SHALL RECEIVE THE FOLLOWING FINISHES:
- INTERIOR EXPOSED SLABS (UNO OR REQUESTED BY THE OWNER): STEEL TROWEL FINISH
 - ALL OTHER CONCRETE: STEEL TROWEL FINISH
12. MAINTAIN CONCRETE AFTER PLACEMENT WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR THE PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE (NOT LESS THAN 7 DAYS). COMPLY WITH THE REQUIREMENTS OF ACI 308- STANDARD PRACTICE FOR CURING CONCRETE. AMERICAN CONCRETE INSTITUTE. A COMBINATION CURING AND SEALING COMPOUND SHALL BE APPLIED AFTER THE CONCRETE HAS BEEN FINISHED OR THE FORMS REMOVED. COMPOUND SHALL MEET THE REQUIREMENTS OF ASTM C1315.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.
5. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
- 5.1 CONCRETE REINFORCEMENT COVER
- EXPOSED TO EARTH OR WEATHER:
- UNFORMED CAST AGAINST EARTH 3" CLEAR
 - FORMED #6 AND LARGER 2" CLEAR
 - FORMED #5 AND SMALLER 1-1/2" CLEAR
- NOT EXPOSED TO EARTH OR WEATHER:
- WALLS 1" CLEAR
 - COLUMNS (TIES) 1-1/2" CLEAR
 - BEAMS/GIRDERS (STIRRUPS) 1-1/2" CLEAR
 - PT BEAMS/GIRDERS (STIRRUPS) 1-1/2" CLEAR
 - SLABS 3/4" CLEAR
- IN AGGRESSIVE ENVIRONMENTS (SEE SPECIFICATIONS FOR DEFINITION):
- WALLS 1-1/2" CLEAR
 - COLUMNS 2" CLEAR
 - BEAMS/GIRDERS 2" CLEAR
 - SLABS: TOP 1-1/2" CLEAR
 - BOTTOM 3/4" CLEAR
6. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
- CONCRETE REINFORCEMENT: CLASS B TENSION LAP
7. ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL CONFORM TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
8. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
9. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS SHOWN OTHERWISE IN DETAILS.

OC	ON CENTER	T&B	TOP AND BOTTOM
OD	OUTSIDE DIAMETER	T/O	TOP OF
OF	OUTSIDE FACE	THK	THICK
OPNG	OPENING	THRU	THROUGH
OPP	OPPOSITE	THP	TYPICAL
OSH	OVERSIZED HOLE		
		UL	UNDERWRITER'S LABORATORIES
PCF	POUNDS PER CUBIC FOOT	UNO	UNLESS NOTED OTHERWISE
PE	PROFESSIONAL ENGINEER		
PERIM	PERIMETER	VERT	VERTICAL
PJP	PREMIXED JOINT FILLER		
PL	PLATE	W	WITH
PLCS	PLACES	W/O	WITHOUT
PLF	POUNDS PER LINEAR FOOT	WP	WORKING POINT
PREFAB	PREFABRICATED	WS	WATERSTOP
PSF	POUNDS PER SQUARE FOOT	WWF	WELDED WIRE FABRIC
PSI	POUNDS PER SQUARE INCH		
PT	POINT		
		R	RADIUS
		RD	ROOF DRAIN
		REF	REFERENCE
		REINFG	REINFORCING
		REQD	REQUIRED
		RET	RETURN
		REV	REVISION
		RO	ROUGH OPENING
		RTU	ROOFTOP UNIT
		SC	SLIP CRITICAL
		SCHED	SCHEDULE
		SECT	SECTION
		SHT	SHEET
		SIM	SIMILAR
		SLS	SLAB
		SPCS	SPACES
		SPEC(S)	SPECIFICATION(S)
		SO	SQUARE
		SSH	STAINLESS STEEL
		STD	STANDARD
		STIF	STIFFENER
		STL	STEEL
		STRUCT	STRUCTURAL
		SYM	SYMMETRICAL



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PROJECT NORTH

Issue	Date & Description	By
03/26/2025	ISSUED FOR CONSTRUCTION	KH/HJ

Drawn by	Author	Reviewed by	Checker
----------	--------	-------------	---------

Seal/Signature



Project:
**GRADY LEGAL HALL HVAC
MODIFICATIONS AND HEALTH EQUITY
SUITE RENOVATION**

Project Number

24.187.03

Sheet Title

STRUCTURAL GENERAL NOTES

Scale

3/4" = 1'-0"

Sheet Number

S00.01

STATEMENT OF SPECIAL INSPECTIONS

PROJECT: GRADY LEGAL HALL HEALTH EQUITY SUITE RENOVATION
LOCATION: 36 JESSE HILL JR DR SE ATLANTA, GEORGIA
PERMIT APPLICANT: JON RICHEY, JON RICHEY@WSP.COM, 470-496-1789, ALTERNATE CONTACT:
TAURIS LIGON, TLI@GONGMIN.EDU, 470-974-4497
APPLICANT ADDRESS: WSP USA, 5346 MCARTHUR ROAD, NE, SUITE 300, ATLANTA, GA 30328
ALTERNATE CONTACT ADDRESS: 80 JESSE HILL JR. DRIVE, SE ATLANTA, GA 30303

ARCHITECT OF RECORD: WILLIAM HART, PA
STRUCTURAL ENGINEER OF RECORD: HOLLY JEFFREYS, PE, SE SHEAR STRUCTURAL
MECHANICAL ENGINEER OF RECORD: QUYEN TU, PE
ELECTRICAL ENGINEER OF RECORD: LEE ROBBERSON, PE
REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE: WILLIAM TODD FREEMAN, PE

This Statement of Special Inspections is submitted in accordance with Section 1704.3 of the 2018 International Building Code. It includes a Schedule of Special Inspection Services applicable to the above-referenced Project as well as the identity of the individuals, agencies, or firms intended to be retained for conducting these inspections. If applicable, it includes Requirements for Seismic Resistance and/or Requirements for Wind Resistance.

Are Requirements for Seismic Resistance included in the Statement of Special Inspections? ☒ N

Are Requirements for Wind Resistance included in the Statement of Special Inspections? ☐ N


The Special Inspector(s) shall keep records of all inspections and shall furnish interim inspection reports to the Building Official and to the Registered Design Professional in Responsible Charge at a frequency agreed upon by the Design Professional and the Building Official prior to the start of work. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge prior to completion of that phase of work. A Final Report of Special Inspections documenting required special inspections and corrections of any discrepancies noted in the inspections shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge at the conclusion of the project.

Frequency of interim report submissions to the Registered Design Professional in Responsible Charge:

☐ Weekly ☒ Bi-Weekly ☐ Monthly Other, specify: _____

The Special Inspection program does not relieve the Contractor of the responsibility to comply with the Contract Documents. Jobsite safety and means and methods of construction are solely the responsibility of the Contractor.

Statement of Special Inspections Prepared by:

Holly Jeffreys
 Type or print name

 Signature SEE TITLEBLOCK _____
 Date _____

Building Official's Acceptance:

Signature _____ Date _____
 Permit Number: _____

Frequency of interim report submissions to the Building Official:

☐ Monthly ☐ Bi- Monthly ☐ Upon Completion Other, specify: _____

Special Inspections for Seismic Resistance

See the Schedule of Special Inspections for inspection and testing requirements

Seismic Design Category: B

Statement of Special Inspection for Seismic Resistance Required (Yes/No): NO

Description of seismic force-resisting system subject to special inspection and testing for seismic resistance:
 (As required per IBC Sections 1705.12.1, 1705.12.2, and 1705.12.3) (Special inspections for seismic resistance of structural steel, where required, shall be in accordance with AISC 341)

Description of designated seismic systems subject to special inspection and testing for seismic resistance:
 (Required for architectural, electrical and mechanical systems and their components that require design in accordance with Chapter 13 of ASCE 7, have a component importance factor, I_p , greater than one and are in Seismic Design Categories C, D, E or F.)

TABLE 1704.2.1 MINIMUM SPECIAL INSPECTOR QUALIFICATIONS			
Category of Testing and Inspection	Minimum Qualifications (refer to key at end of Table)		
	Shop Testing or Inspection	Field Testing or Inspection	Review Testing, Certification & Lab Reports
1704.2.5 Inspection of Fabricators			
Structural steel construction	C, F, G		
1705.2, 1705.10, 1705.11 & 1705.12 Steel Construction			
Verification of welding consumables, filler metals, procedure specifications, procedure qualification records and personnel performance qualification records			C, F
Nondestructive testing of welding	G	G	
Inspection of welding	C, F	C, F	
Verification of fabricator and erector documents as listed in AISC 360, chapter N, paragraph 3.2			A, C
Material verification of weld filler materials			C, F
Inspection of high strength bolting and steel frame joint details		A, C	
Inspection of embedment		A, C, F	
1705.10, 1705.11 & 1705.12 Seismic and Wind Resistance			
Periodic inspection of fabrication, installation and/or anchorage of building systems and components		A	

KEY:

- A. Georgia Professional Engineer (GA PE) competent in the specific task area or graduate of accredited engineering/engineering technology program under the direct supervision of a GA PE.
- B. Georgia Registered Architect (GA RA) or graduate of accredited architecture/architecture technology program under the direction of a GA RA
- C. International Code Council (ICC) Special Inspector Certification specific to the particular material and testing methodology applicable to each Category of Testing and Inspection listed in the table.
- D. Post-tensioning Institute (PTI) Certification, Level 2, bonded or unbonded as applicable.
- E. Pre-stressed Concrete Institute (PCI) Certified Inspector.
- F. American Welding Society (AWS) Certified Welding Inspector (CWI) or AWS Certified Associate Welding Inspector working under the direct on-site supervision of a CWI.
- G. American Society for Nondestructive Testing (ASNT) Level II certification, or a Level III certification if previously certified as a Level II in the particular material and testing methodology applicable to each Category of Testing and Inspection listed in the table.
- H. American Concrete Institute (ACI) Concrete Construction Special Inspector.
- I. National Institute for Certification in Engineering Technologies (NICET) Level II or higher certification specific to the particular material and testing methodology applicable to each Category of Testing and Inspection listed in the table.
- J. ACI Concrete Field Testing Technician with Grade 1 certification.
- K. Georgia Concrete and Products Association (GC&PA) – Masonry Association of Georgia (MAG) Masonry Construction Inspector Certification.
- L. National Concrete Masonry Association (NCMA) Concrete Masonry Testing Procedures certification.
- M. GC&PA – MAG Masonry Testing Technician certification.
- N. NICET Certified Engineering Technologist (CT).
- O. Other Qualified Special Inspector as approved by the Building Official.
- P. American Concrete Institute (ACI) Strength Testing Technician.

Notes:

- The Special Inspector shall meet one of the minimum qualifications listed for the applicable Category of Testing and Inspection.
- Materials testing shall be done by an Approved Testing Agency meeting the requirements of IBC Section 1703 and ASTM E 329.

SCHEDULE OF SPECIAL INSPECTION SERVICES				
PROJECT		GRADY LEGAL HALL HEALTH EQUITY SUITE RENOVATION		
MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
		Y/N	EXTENT	AGENT* DATE COMPLETED
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements - add additional rows as needed.)	Submittal review, shop (3) and/or field inspection			N
1. Inspected of anchors post-installed in solid grouted masonry. Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, masonry unit, grout, masonry compressive strength, anchor embedment and tightening torque	Field inspection	N	Periodic or as required by the research report issued by an approved source	
2. Aggregate Pier Inspection: The special inspector's responsibilities include, but are not limited to, review of the aggregate pier designer's use of soil parameters as presented in the project soils report, and during construction, verification of aggregate properties, type and number of lifts of aggregate, hole size and depths and top elevations of the pier elements, and applied energy. Additionally, results of qualitative tests on production aggregate pier elements such as modulus load testing, uplift pull-out testing, bottom stabilization tests and dynamic cone penetration tests, shall be reviewed to verify compliance with design specifications	Field inspection	N	Periodic or as required by the research report issued by an approved source	
1705.2.1 Structural Steel Construction				
1. Fabricator and erector documents (Verified reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	Y	Each submittal	
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic	
3. Structural steel welding: a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Y	Observe (4)	
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	
d. Nondestructive testing (NDT) of welded joints: see <i>Commentary</i>				
1) Complete penetration groove welds 5/16" or greater in risk category III or IV	Shop (3) or field ultrasonic testing - 100%	N	Periodic	
2) Complete penetration groove welds 5/16" or greater in risk category II	Shop (3) or field ultrasonic testing - 10% of welds minimum	N	Periodic	
3) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing	N	Periodic	
4) Fabricator's NDT reports when fabricator performs NDT	Verify reports	Y	Each submittal (5)	
4. Structural steel bolting:	Shop (3) and field inspection			
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Y	Observe or Perform as noted (4)	
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Y	Observe (4)	
1) Pre-tensioned and slip-critical joints		N		
a) Turn-of-nut with matching markings		N	Periodic	
b) Direct tension indicator		N	Periodic	
c) Twist-of type tension control bolt		N	Periodic	
d) Turn-of-nut without matching markings		N	Continuous	
e) Calibrated wrench		N	Continuous	
2) Snug-tight joints		Y	Periodic	
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Y	Perform (4)	
5. Visual inspection of exposed cut surfaces of galvanized structural steel main members and exposed corners of the rectangular HSS for cracks subsequent to galvanizing	Shop (3) and field inspection and testing	N	Periodic	
6. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Periodic	
7. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Y	Periodic	
1705.3 Concrete Construction				
1. Inspection and placement verification of reinforcing steel and prestressing tendons.	Shop (3) and field inspection	Y	Periodic	
2. Reinforcing bar welding:		N		
a. Verification of weldability of bars other than ASTM A706.		N	Periodic	
b. Inspection of single-pass fillet welds 5/16 or less in size.		N	Periodic	
c. Inspection of all other welds.		N	Continuous	
3. Inspection of anchors cast in concrete.	Shop (3) and field inspection	Y	Periodic	
4. Inspection of anchors post-installed in hardened concrete members per research reports, or, if no specific requirements are provided, requirements shall be provided by the registered design professional and approved by the building official, including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Y	Periodic or as required by the research report issued by an approved source	
a. Adhesive anchors installed in horizontal or upward-inclined orientation that resist sustained tension loads		N	Continuous	
b. Mechanical and adhesive anchors not defined in 4a.		Y	Periodic	

SCHEDULE OF SPECIAL INSPECTION SERVICES					
PROJECT		GRADY LEGAL HALL HEALTH EQUITY SUITE RENOVATION			
MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT			
		Y/N	EXTENT	AGENT*	DATE COMPLETED
5. Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic		
6. Prior to placement, fresh concrete sampling, perform slump and air content tests and determine temperature of concrete and perform any other tests as specified in construction documents.	Shop (3) and field inspection	Y	Continuous		
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous		
8. Verify maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Y	Periodic		
9. Inspection of prestressed concrete:	Shop (3) and field inspection	N			
a. Application of prestressing force		N	Continuous		
b. Grouting of bonded prestressing tendons		N	Continuous		
10. Inspect erection of precast concrete members		N	Periodic		
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	N	Periodic		
12. Inspection of formwork for shape, levels, location and dimensions	Field inspection	Y	Periodic		
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic		
1705.12.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance					
1. Inspection during the anchorage of electrical equipment for emergency or standby power systems in SDC C, D, E or F	Field inspection	N	Periodic		
2. Inspection during the anchorage of other electrical equipment in SDC E or F	Field inspection	N	Periodic		
3. Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units in SDC C, D, E or F	Field inspection	N	Periodic		
4. Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials SDC C, D, E or F	Field inspection	N	Periodic		
5. Inspection during the installation and anchorage of vibration isolation systems in SDC C, D, E or F where nominal clearance of 1/4 inch or less is required by the approved construction documents	Field inspection	N	Periodic		
6. Inspection during installation of mechanical and electrical equipment, including duct work, piping systems and their structural supports, where automatic fire sprinkler systems are installed in structures assigned to SDC C, D, E, or F to verify one of the following clearances are used:					
a. ASCE/SEI 7, Section 13.2.3 minimum required clearances have been provided;	Field inspection	N	Periodic		
b. A three inch or greater nominal clearance has been provided between fire protection sprinkler system drops and spigots and structural members not used collectively or independently to support the sprinklers, equipment attached to the building structure, and other systems' piping.	Field inspection	N	Periodic		
1705.12.7 Storage Racks Special Inspections for Seismic Resistance					
Inspection during the anchorage of storage racks 8 feet or greater in height in structures assigned to SDC D, E or F.	Field inspection	N	Periodic		
1705.12.8 Seismic Isolation Systems					
Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system in structures assigned to SDC B, C, D, E or F.	Shop and field inspection	N	Periodic		
* INSPECTION AGENTS		ADDRESS		TELEPHONE NO.	
FIRM					
1. UNITED CONSULTING		625 HOLCOMB BRIDGE RD. NORCROSS, GA		(770) 209-0029	
2. GEOHYDRO ENGINEERS		1000 COBB PL BLVD #290, KENNESAW, GA		(770) 426-7100	
3. TERRACON		2105 NEWPOINT PL #600, LAWRENCEVILLE, GA		(770) 623-0755	
Notes:					
1. The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.					
2. The list of Special Inspectors may be submitted as a separate document, if noted so above.					
3. Special Inspections as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2.					
4. Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks as they occur.					
5. NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.					
Are Requirements for Seismic Resistance included in the Statement of Special Inspections?				N	
Are Requirements for Wind Resistance included in the Statement of Special Inspections?				N	
				DATE: SEE TITLE BLOCK	



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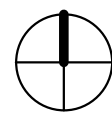
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PROJECT NORTH

 Issue	Date & Description	By
03/26/2025	ISSUED FOR CONSTRUCTION	KH/HJ

Drawn by **Author** Reviewed by **Checker**

Seal/Signatur



Project

**GRADY LEGAL HALL HVAC
MODIFICATIONS AND HEALTH EQUITY
SUITE RENOVATION**

Project Number

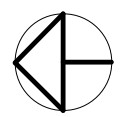
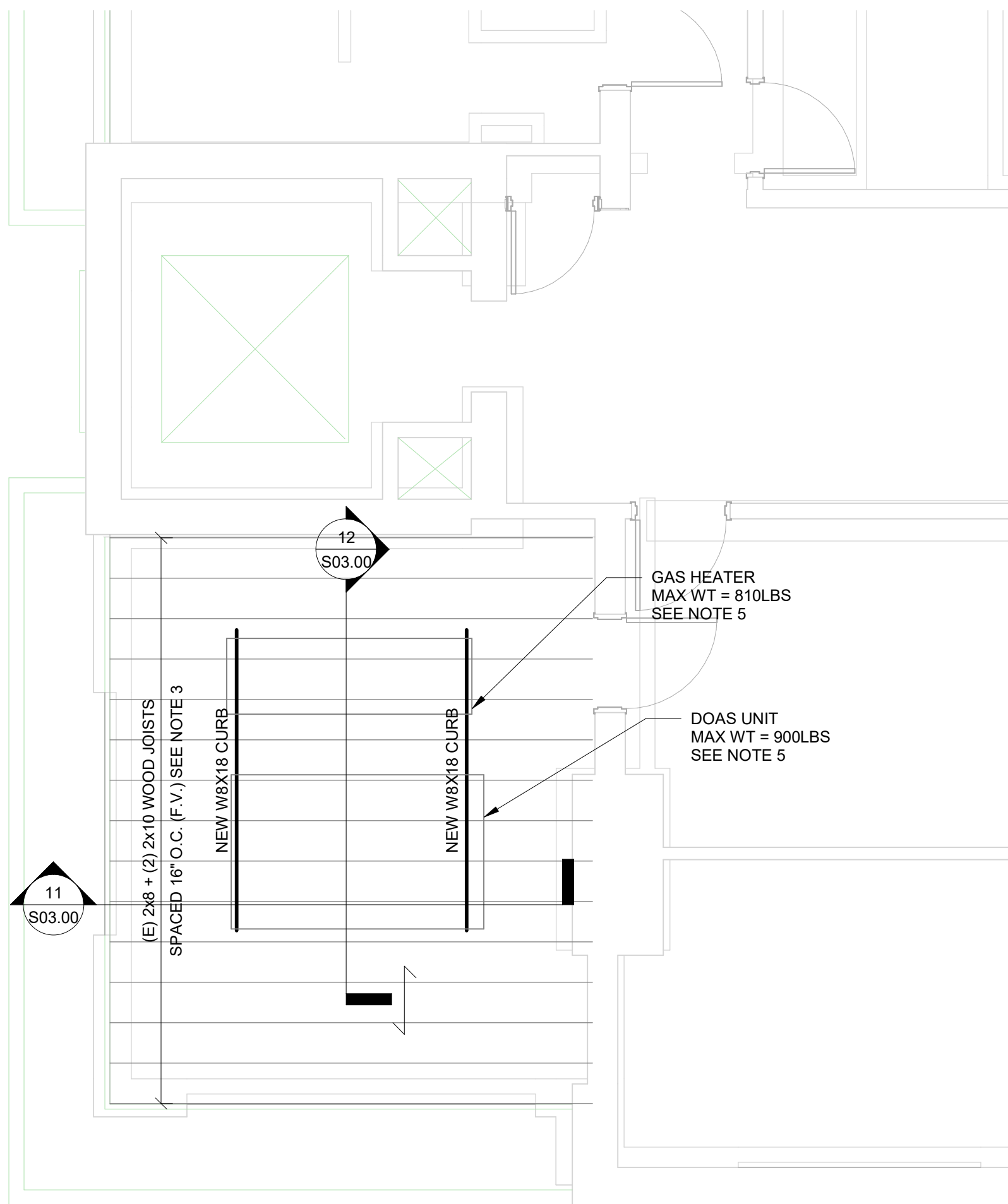
Sheet Title
SPECIAL INSPECTIONS

Scale

3/4" = 1'-0"

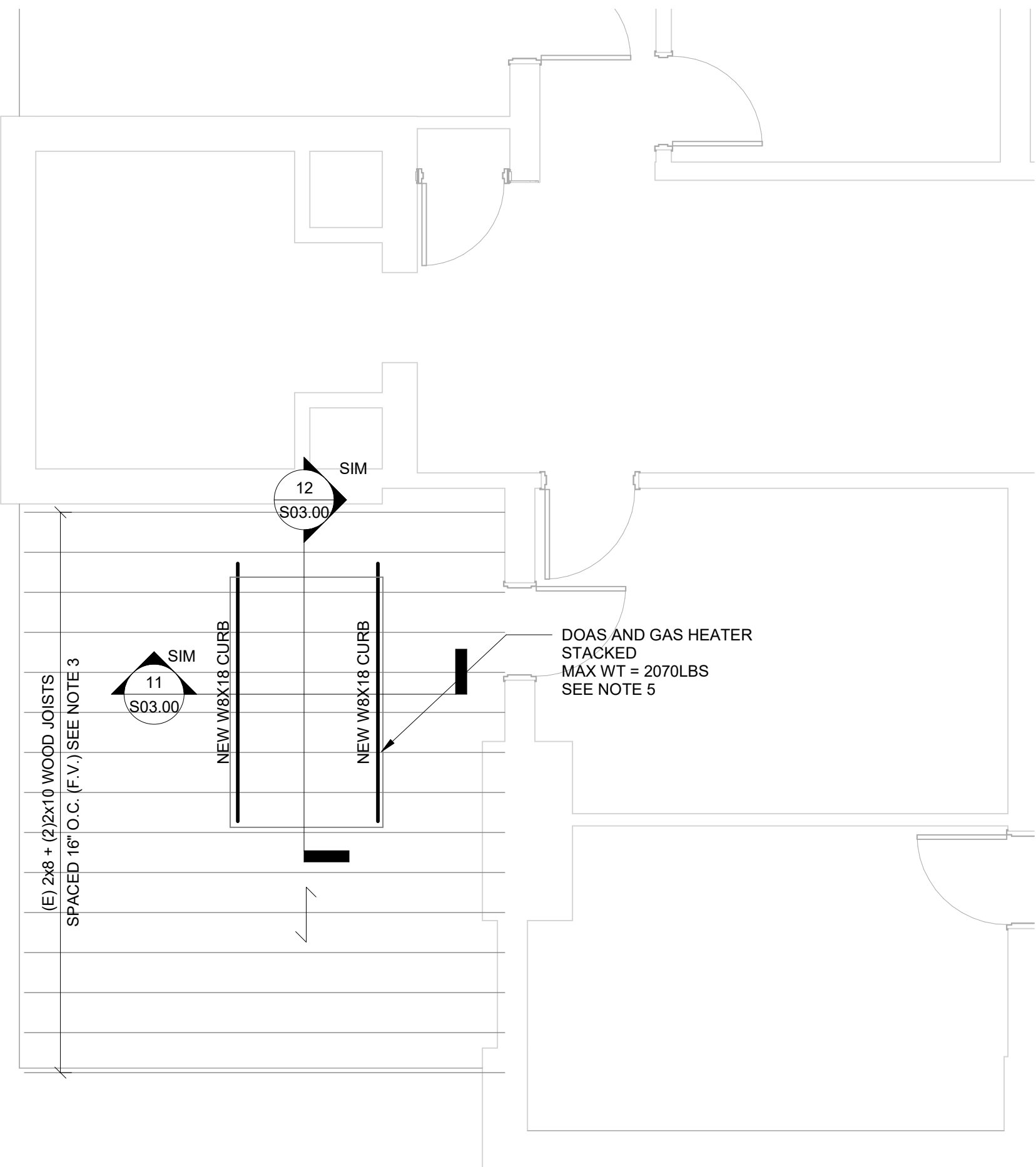
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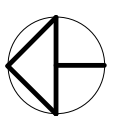
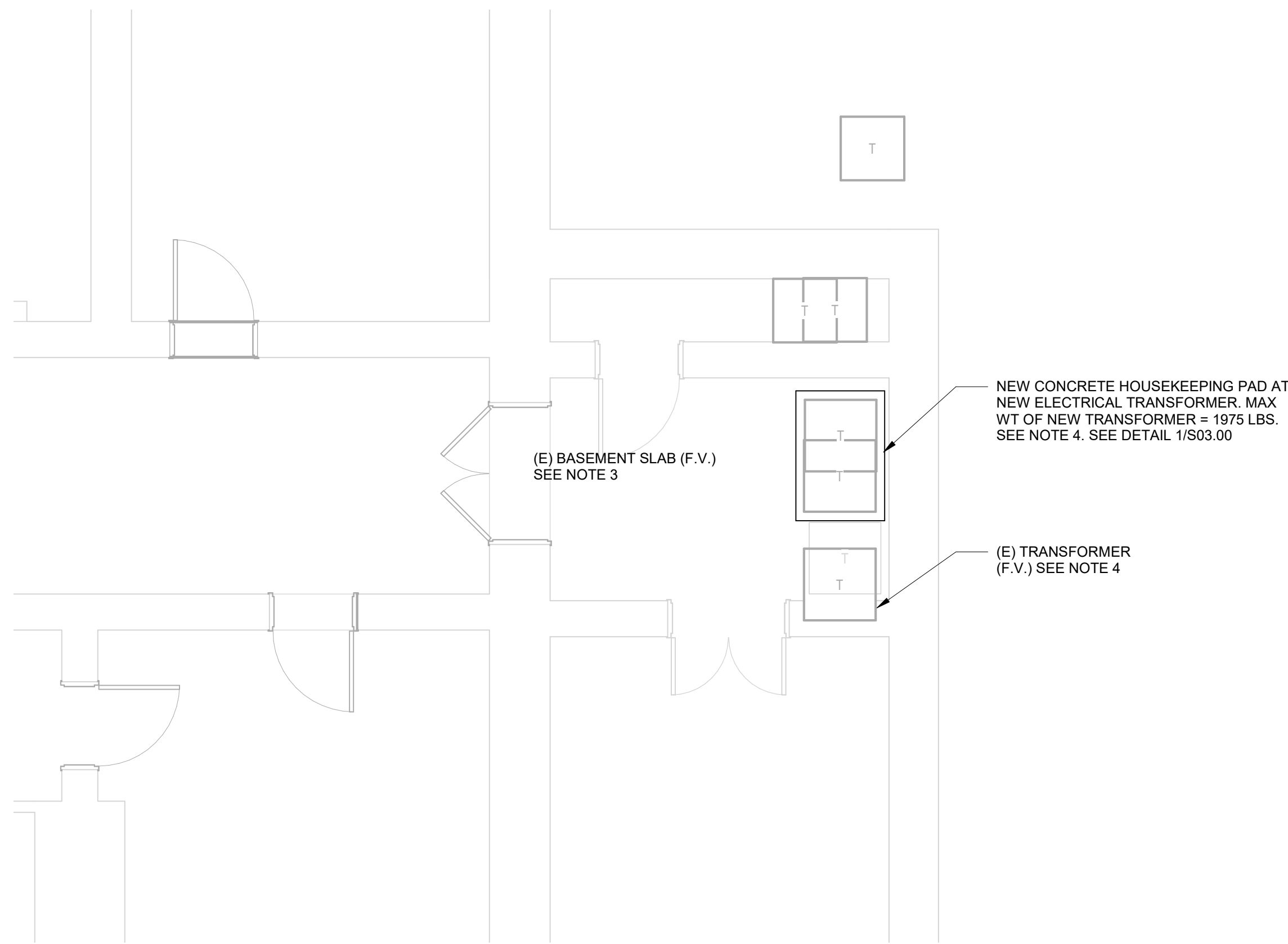
1 PARTIAL FOURTH FLOOR FRAMING/ATTIC PLAN
S01.00 1/4" = 1'-0"

- NOTES:
- SEE S00.01 FOR STRUCTURAL GENERAL NOTES.
 - SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
 - (E) 2x8 WOOD FLOOR FRAMING WAS PREVIOUSLY SISTERED WITH (2)2x10 FOR AN HVAC MODIFICATION PROJECT. (EX STRUCTURAL DRAWINGS DATED 03.18.2005)
 - INDICATES SPAN DIRECTION OF (E) 3/4" PLYWOOD FLOOR SHEATHING (F.V.)
 - EX MECH AHU TO BE REMOVED AND REPLACED. SEE MECH FOR EXACT SIZE AND LOCATION OF NEW MECHANICAL EQUIPMENT.
 - FOR NEW WALL OPENINGS, SEE LINTEL SCHEDULE 4/S03.00.



2 PARTIAL ALTERNATE FOURTH FLOOR FRAMING/ATTIC PLAN
S01.00 1/4" = 1'-0"

- NOTES:
- SEE S00.01 FOR STRUCTURAL GENERAL NOTES.
 - SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
 - (E) 2x8 WOOD FLOOR FRAMING WAS PREVIOUSLY SISTERED WITH (2)2x10 FOR AN HVAC MODIFICATION PROJECT. (EX STRUCTURAL DRAWINGS DATED 03.18.2005)
 - INDICATES SPAN DIRECTION OF (E) 3/4" PLYWOOD FLOOR SHEATHING (F.V.)
 - EX MECH AHU TO BE REMOVED AND REPLACED. SEE MECH FOR EXACT SIZE AND LOCATION OF NEW MECHANICAL EQUIPMENT.
 - FOR NEW WALL OPENINGS, SEE LINTEL SCHEDULE 4/S03.00.



3 PARTIAL BASEMENT PLAN
S01.00 1/4" = 1'-0"

- NOTES:
- SEE S00.01 FOR STRUCTURAL GENERAL NOTES.
 - SEE ARCH FOR ADDITIONAL INFORMATION AND DIMENSIONS.
 - (E) 4" CONCRETE SLAB ON GRADE (EX ARCHITECTURAL DRAWINGS DATED 01.03.1990)
 - SEE ELEC FOR EXACT SIZE AND LOCATION OF EX AND NEW ELECTRICAL EQUIPMENT.



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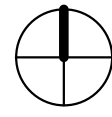
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PROJECT NORTH

Issue	Date & Description	By
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Drawn by Author Reviewed by Checker

Seal/Signature



Project:
GRADY LEGAL HALL HVAC
MODIFICATIONS AND HEALTH EQUITY
SUITE RENOVATION

Project Number

24.187.03

Sheet Title

STRUCTURAL PARTIAL PLANS

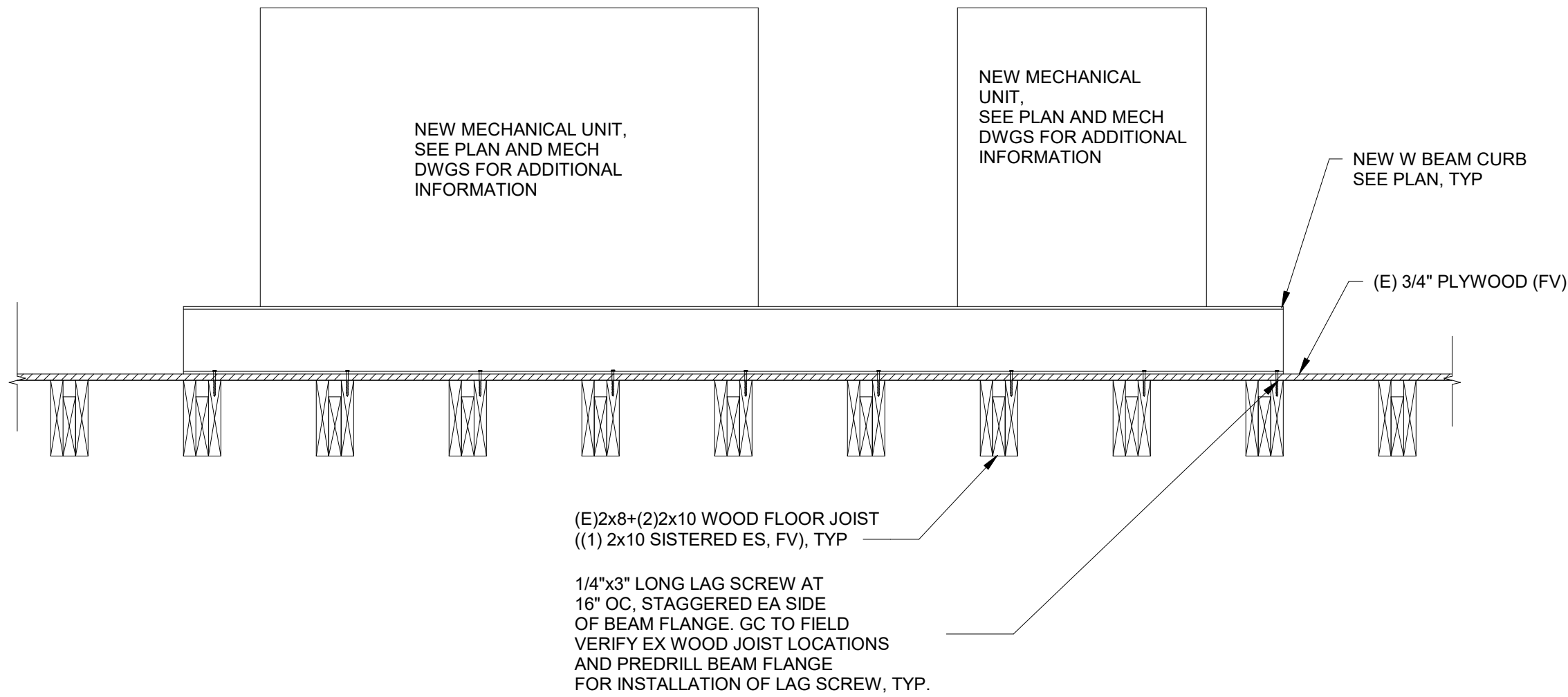
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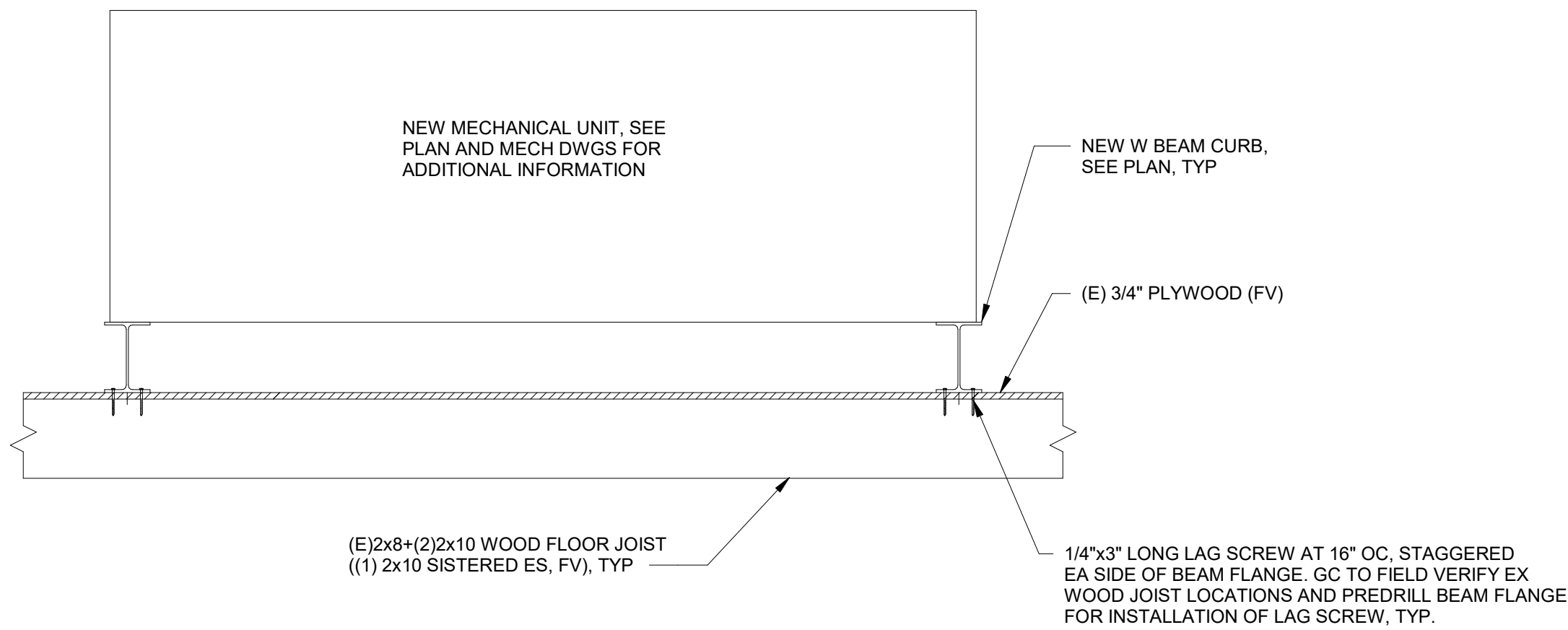
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12 SECTION AT NEW MECHANICAL UNITS
3/4" = 1'-0"

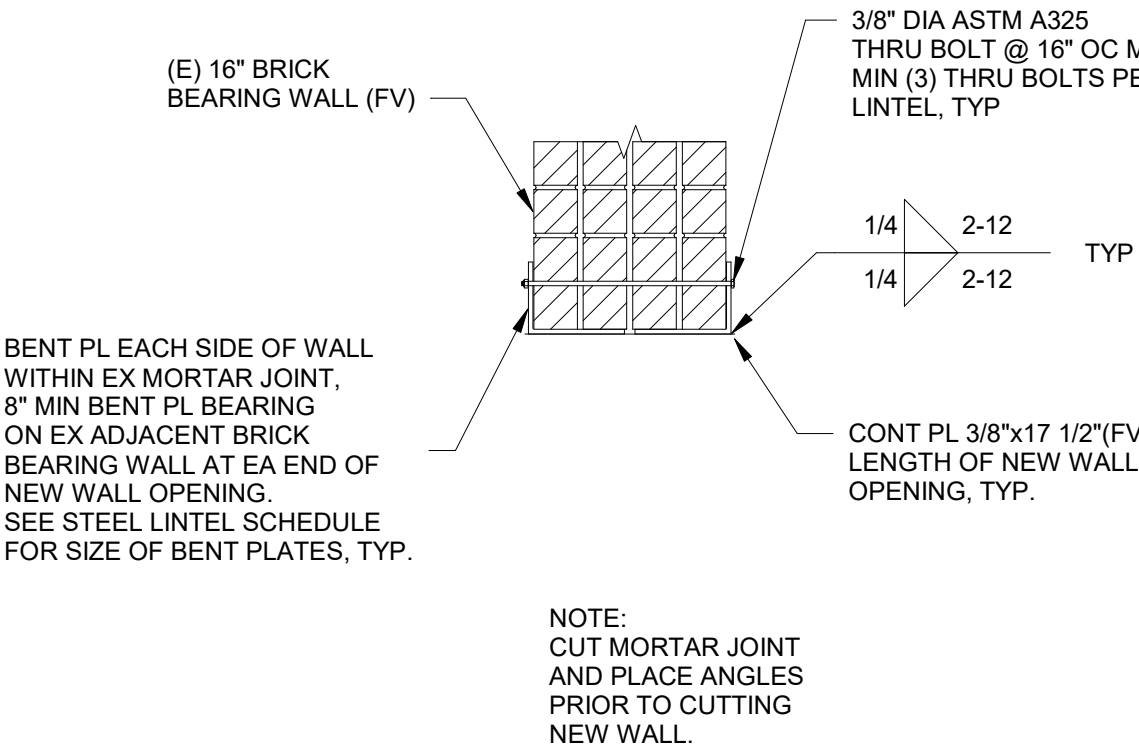


11 SECTION AT NEW MECHANICAL UNITS (EAST WEST)
3/4" = 1'-0"

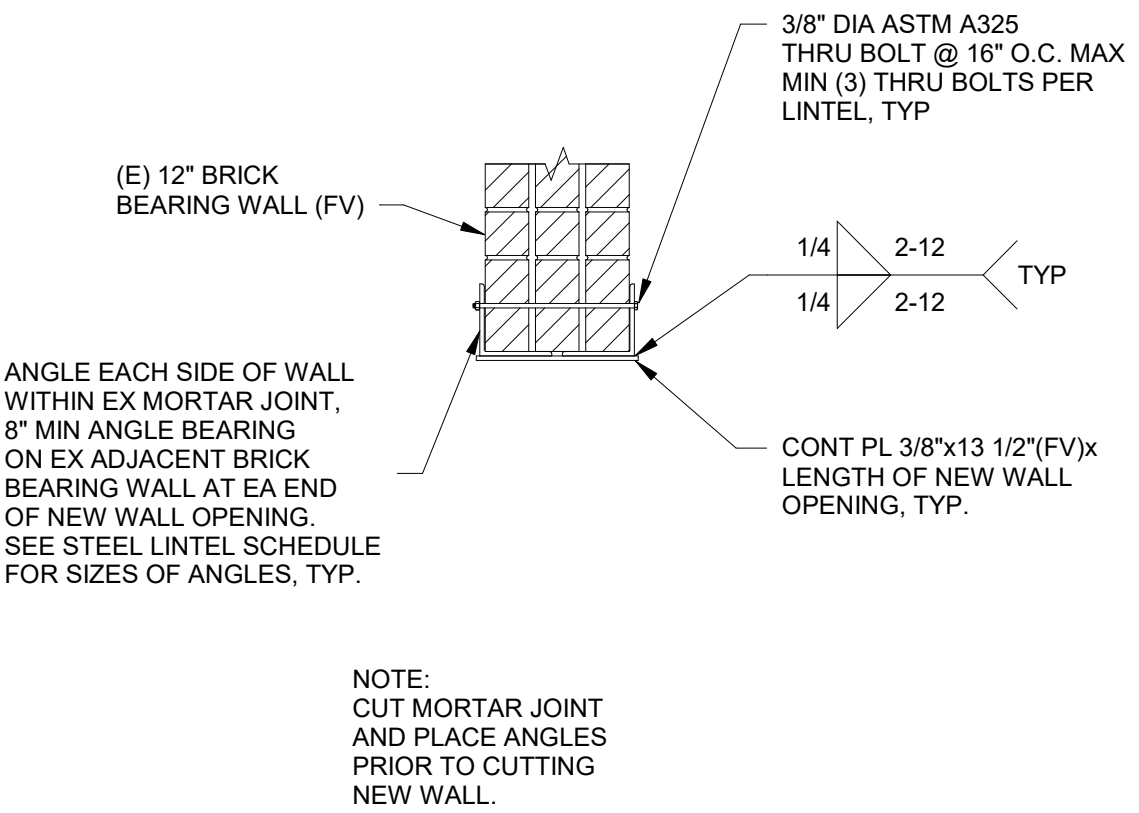
STANDARD STEEL LINTEL SCHEDULE			
MAXIMUM OPENING WIDTH	WALL THK	LINTEL TYPE	REMARKS
3'-6"	12"	(2) L6x6x3/8 + 3/8" BOTTOM PLATE	SEE DETAIL 2/S03.00
3'-6"	16"	(2) B PL 3/8"x8"x6" (LLH) 3/8" BOTTOM PLATE	SEE DETAIL 3/S03.00

NOTES:
1. SEE MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF NEW WALL OPENINGS, REQUIRING STEEL LINTELS, FOR MECHANICAL DUCTWORK.

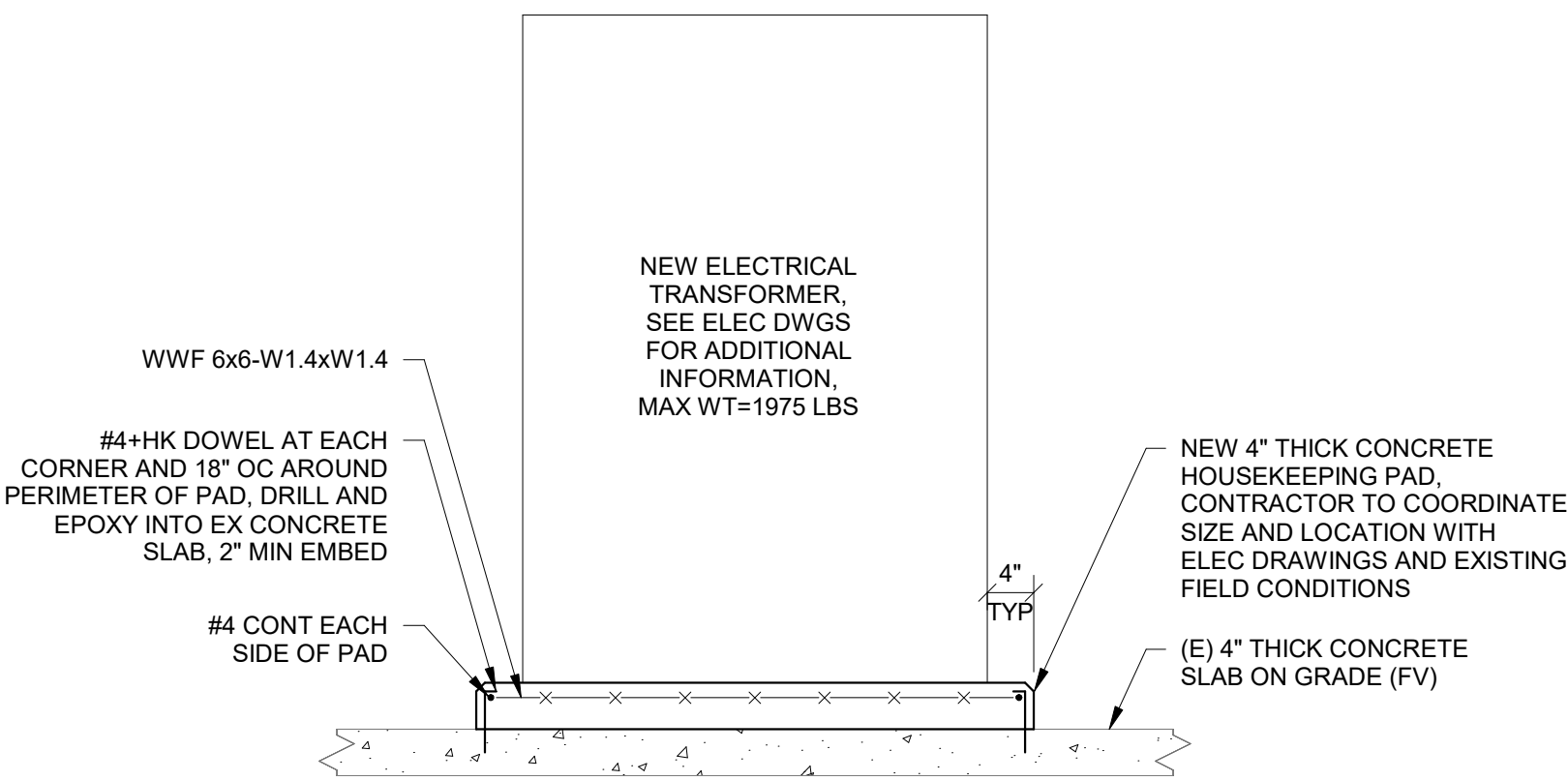
4 STEEL LINTEL SCHEDULE
3/4" = 1'-0"



3 TYPICAL STEEL LINTEL DETAIL (16" BRICK BEARING WALL)
3/4" = 1'-0"



2 TYPICAL STEEL LINTEL DETAIL
3/4" = 1'-0"



1 CONCRETE HOUSEKEEPING PAD AT NEW ELEC TRANSFORMER
3/4" = 1'-0"



WSP USA Buildings Inc.
3340 Peachtree Road, NE,
Suite 1100
Atlanta, GA 30326
(404) 815-4152
wsp.com



1401 Peachtree St.
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Atlanta, GA 30308
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Shearstructural.com

Grady Legal Hall HVAC Modifications and
Health Equity Suite Renovation
36 Jesse Hill Jr. Dr. SE
Atlanta, Georgia



PROJECT NORTH

Issue	Date & Description	By
03/26/2025	ISSUED FOR CONSTRUCTION	KH/HJ

Drawn by Author Reviewed by Checker

Seal/Signature



Project
GRADY LEGAL HALL HVAC
MODIFICATIONS AND HEALTH EQUITY
SUITE RENOVATION

Project Number

24.187.03

Sheet Title

STRUCTURAL SECTIONS AND DETAILS

Scale

3/4" = 1'-0"

Sheet Number

S03.00

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